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A posteriori error analysis for Kirchhoff plate elements

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Outline

Kirchhoff plate bending model

Finite element formulations

- ▶ Morley element
- ▶ Stabilized C^0 -element

A posteriori error estimates

Numerical results

Conclusions and references

Kirchhoff plate bending model

Displacement formulation. Find the deflection w such that, in the domain $\Omega \subset \mathbb{R}^2$, it holds

$$\frac{1}{6(1-\nu)} \Delta^2 w = f.$$

Mixed formulation. Find the deflection w , rotation β and the shear stress \mathbf{q} such that it holds

$$-\operatorname{div} \mathbf{q} = f,$$

$$\operatorname{div} \mathbf{m}(\beta) + \mathbf{q} = \mathbf{0}, \quad \text{with} \quad \mathbf{m}(\beta) = \frac{1}{6} \left\{ \boldsymbol{\varepsilon}(\beta) + \frac{\nu}{1-\nu} \operatorname{div} \beta \mathbf{I} \right\},$$

$$\nabla w - \beta = \mathbf{0}.$$

- Furthermore, the boundary conditions on the clamped, simply supported and free boundaries Γ_C , Γ_S and Γ_F are imposed.

FE formulations — Morley element

- We define the **discrete space** for the **deflection** as follows:

$$W_h = \left\{ v \in M_{2,h} \mid \int_E \llbracket \frac{\partial v}{\partial \mathbf{n}_E} \rrbracket = 0 \quad \forall E \in \mathcal{E}_h \right\},$$

where E represents an edge of a triangle K in a triangulation \mathcal{T}_h , and $M_{2,h}$ denotes the space of the **second order** piecewise polynomial functions on \mathcal{T}_h which are

- continuous at the vertices of all the internal triangles and
- zero at all the triangle vertices on the clamped boundary.

Finite element method. Find $w_h \in W_h$ such that

$$\sum_{K \in \mathcal{T}_h} (\mathbf{E}\boldsymbol{\varepsilon}(\nabla w_h), \boldsymbol{\varepsilon}(\nabla v))_K = (f, v) \quad \forall v \in W_h.$$

Stabilized C^0 -element

- Given an integer $k \geq 1$, we define the **discrete spaces** for the **deflection** and the **rotation**, respectively, as

$$W_h = \{v \in W \mid v|_K \in P_{k+1}(K) \quad \forall K \in \mathcal{T}_h\},$$

$$\mathbf{V}_h = \{\boldsymbol{\eta} \in \mathbf{V} \mid \boldsymbol{\eta}|_K \in [P_k(K)]^2 \quad \forall K \in \mathcal{T}_h\},$$

where $P_k(K)$ denotes the polynomial space of **degree** k on K .

Finite element method. Find $(w_h, \boldsymbol{\beta}_h) \in W_h \times \mathbf{V}_h$ such that

$$\mathcal{A}_h(w_h, \boldsymbol{\beta}_h; v, \boldsymbol{\eta}) = (f, v) \quad \forall (v, \boldsymbol{\eta}) \in W_h \times \mathbf{V}_h,$$

where the bilinear form \mathcal{A}_h we split as

$$\mathcal{A}_h(z, \boldsymbol{\phi}; v, \boldsymbol{\eta}) = \mathcal{B}_h(z, \boldsymbol{\phi}; v, \boldsymbol{\eta}) + \mathcal{D}_h(z, \boldsymbol{\phi}; v, \boldsymbol{\eta}),$$

with the *stabilized* (α) *bending* part (R–M with the limit $t \rightarrow 0$)

$$\begin{aligned} \mathcal{B}_h(z, \boldsymbol{\phi}; v, \boldsymbol{\eta}) &= (\mathbf{m}(\boldsymbol{\phi}), \boldsymbol{\varepsilon}(\boldsymbol{\eta})) - \sum_{K \in \mathcal{T}_h} \alpha h_K^2 (\mathbf{L}\boldsymbol{\phi}, \mathbf{L}\boldsymbol{\eta})_K \\ &+ \sum_{K \in \mathcal{T}_h} \frac{1}{\alpha h_K^2} (\nabla z - \boldsymbol{\phi} - \alpha h_K^2 \mathbf{L}\boldsymbol{\phi}, \nabla v - \boldsymbol{\eta} - \alpha h_K^2 \mathbf{L}\boldsymbol{\eta})_K \end{aligned}$$

and the *stabilized* (γ) *free boundary* part

$$\begin{aligned} \mathcal{D}_h(z, \boldsymbol{\phi}; v, \boldsymbol{\eta}) &= \langle m_{ns}(\boldsymbol{\phi}), [\nabla v - \boldsymbol{\eta}] \cdot \mathbf{s} \rangle_{\Gamma_F} + \langle [\nabla z - \boldsymbol{\phi}] \cdot \mathbf{s}, m_{ns}(\boldsymbol{\eta}) \rangle_{\Gamma_F} \\ &+ \sum_{E \in \mathcal{F}_h} \frac{\gamma}{h_E} \langle [\nabla z - \boldsymbol{\phi}] \cdot \mathbf{s}, [\nabla v - \boldsymbol{\eta}] \cdot \mathbf{s} \rangle_E \end{aligned}$$

for all $(z, \boldsymbol{\phi}) \in W_h \times \mathbf{V}_h$, $(v, \boldsymbol{\eta}) \in W_h \times \mathbf{V}_h$, where \mathcal{F}_h represents the collection of all the boundary edges on the free boundary Γ_F , and the twisting moment is $m_{ns} = \mathbf{s} \cdot \mathbf{m}\mathbf{n}$.

- The first term in \mathcal{D}_h is for *consistency*, the second one for *symmetry* and the last one for *stability*.

A posteriori error estimates

- ▶ We use the following **notation**: $[[\cdot]]$ for jumps, h_E and h_K for the edge length and the element diameter.

Interior error indicators

- ▶ For the **local error indicator** η_K we define: for all the **elements** K in the mesh \mathcal{T}_h , and for all the **internal edges** $E \in \mathcal{I}_h$,

$$\text{(Morley)} \quad \tilde{\eta}_K^2 := h_K^4 \|f\|_{0,K}^2,$$

$$\text{(Stabil.)} \quad \tilde{\eta}_K^2 := h_K^4 \|f + \operatorname{div} \mathbf{q}_h\|_{0,K}^2 + h_K^{-2} \|\nabla w_h - \boldsymbol{\beta}_h\|_{0,K}^2,$$

$$\text{(Morley)} \quad \eta_E^2 := h_E^{-3} \|[[w_h]]\|_{0,E}^2 + h_E^{-1} \left\| \left[\frac{\partial w_h}{\partial \mathbf{n}_E} \right] \right\|_{0,E}^2,$$

$$\text{(Stabil.)} \quad \eta_E^2 := h_E^3 \|[[\mathbf{q}_h \cdot \mathbf{n}]]\|_{0,E}^2 + h_E \left\| \left[\mathbf{m}(\boldsymbol{\beta}_h) \mathbf{n} \right] \right\|_{0,E}^2.$$

Boundary error indicators

- ▶ Let the **boundary** $\partial\Omega$ of the plate be divided into the parts of the different boundary conditions: **clamped**, **simply supported** and **free**, i.e., $\partial\Omega = \Gamma_C \cup \Gamma_S \cup \Gamma_F$.
- ▶ For the **Morley** element, we assume that $\partial\Omega = \Gamma_C$ and for the edges on the **clamped** boundary Γ_C

$$\text{(Morley)} \quad \eta_{E,C}^2 = h_E^{-3} \| \llbracket w_h \rrbracket \|_{0,E}^2 + h_E^{-1} \| \llbracket \frac{\partial w_h}{\partial \mathbf{n}_E} \rrbracket \|_{0,E}^2 .$$

- ▶ For the **stabilized** C^0 -element, for the edges on the **simply supported** boundary Γ_S

$$\text{(Stabil.)} \quad \eta_{E,S}^2 := h_E \| m_{nn}(\boldsymbol{\beta}_h) \|_{0,E}^2 ,$$

and for the edges on the **free** boundary Γ_F

$$\text{(Stabil.)} \quad \eta_{E,F}^2 := h_E \| m_{nn}(\boldsymbol{\beta}_h) \|_{0,E}^2 + h_E^3 \| \frac{\partial}{\partial s} m_{ns}(\boldsymbol{\beta}_h) - \mathbf{q}_h \cdot \mathbf{n} \|_{0,E}^2 .$$

Error indicators — local and global

- Now, for any element $K \in \mathcal{T}_h$, let the **local** error indicator be

$$\eta_K := \left(\tilde{\eta}_K^2 + \frac{1}{2} \sum_{\substack{E \in \mathcal{I}_h \\ E \subset \partial K}} \eta_E^2 + \sum_{\substack{E \in \mathcal{C}_h \\ E \subset \partial K}} \eta_{E,C}^2 + \sum_{\substack{E \in \mathcal{S}_h \\ E \subset \partial K}} \eta_{E,S}^2 + \sum_{\substack{E \in \mathcal{F}_h \\ E \subset \partial K}} \eta_{E,F}^2 \right)^{1/2},$$

with the **notation**

- \mathcal{I}_h for the collection of all the **internal** edges,
- \mathcal{C}_h , \mathcal{S}_h and \mathcal{F}_h for the collections of all the **boundary** edges on Γ_C , Γ_S and Γ_F , respectively.

- Finally, the **global** error indicator is defined as

$$\eta := \left(\sum_{K \in \mathcal{T}_h} \eta_K^2 \right)^{1/2}.$$

Upper bounds — Reliability

- With \mathcal{E}_h denoting the collection of all the triangle edges, we define the **mesh dependent norms** for the **Morley** element and for the **stabilized** C^0 -element, respectively, as

$$\| \|v\| \|_h^2 := \sum_{K \in \mathcal{T}_h} |v|_{2,K}^2 + \sum_{E \in \mathcal{E}_h} h_E^{-3} \| [v] \|_{0,E}^2 + \sum_{E \in \mathcal{E}_h} h_E^{-1} \| \left[\frac{\partial v}{\partial \mathbf{n}_E} \right] \|_{0,E}^2 ,$$

$$\begin{aligned} \| \|(v, \boldsymbol{\eta})\| \|_h^2 &:= \sum_{K \in \mathcal{T}_h} |v|_{2,K}^2 + \|v\|_1^2 + \sum_{E \in \mathcal{I}_h} h_E^{-1} \| \left[\frac{\partial v}{\partial \mathbf{n}_E} \right] \|_{0,E}^2 \\ &\quad + \sum_{K \in \mathcal{T}_h} h_K^{-2} \| \nabla v - \boldsymbol{\eta} \|_{0,K}^2 + \| \boldsymbol{\eta} \|_1^2 . \end{aligned}$$

Theorem. *There exists positive constants C such that*

$$\text{(Morley)} \quad \| \|w - w_h\| \|_h \leq C\eta ,$$

$$\text{(Stabil.)} \quad \| \|(w - w_h, \boldsymbol{\beta} - \boldsymbol{\beta}_h)\| \|_h + \| \mathbf{q} - \mathbf{q}_h \|_{-1,*} \leq C\eta .$$

Lower bounds — Efficiency

- Let ω_K be the collection of all the triangles in \mathcal{T}_h with a nonempty intersection with the element K .

Theorem. *There exists positive constants C such that*

$$\text{(Morley)} \quad \eta_K \leq C \left(\|w - w_h\|_{h,K} + h_K^2 \|f - f_h\|_{0,K} \right),$$

$$\text{(Stabil.)} \quad \eta_K \leq C \left(\| (w - w_h, \boldsymbol{\beta} - \boldsymbol{\beta}_h) \|_{h,\omega_K} + h_K^2 \|f - f_h\|_{0,\omega_K} \right. \\ \left. + \|\mathbf{q} - \mathbf{q}_h\|_{-1,*,\omega_K} \right),$$

for any element $K \in \mathcal{T}_h$.

Numerical results

- ▶ We have **implemented** the methods in the open-source finite element software *Elmer* developed by CSC – the Finnish IT Center for Science.
- ▶ Test problems with **convex rectangular domains**, and with known exact solutions, we have used for investigating the **effectivity index** for the error estimators derived.
- ▶ **Non-convex domains** we have used for studying the **adaptive** performance and robustness of the methods.

Effectivity index

(Morley) $\iota = \frac{\eta}{\|w - w_h\|_h}$

(Stabil.) $\iota = \frac{\eta}{\|(w - w_h, \beta - \beta_h)\|_h}$

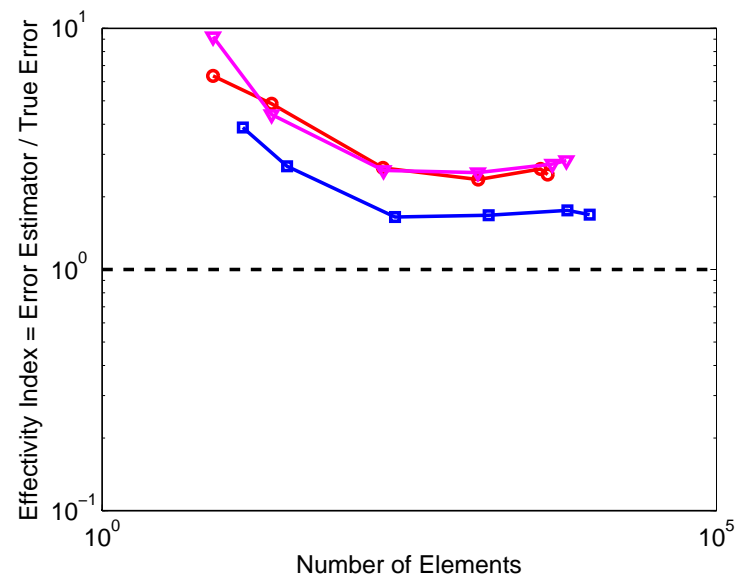
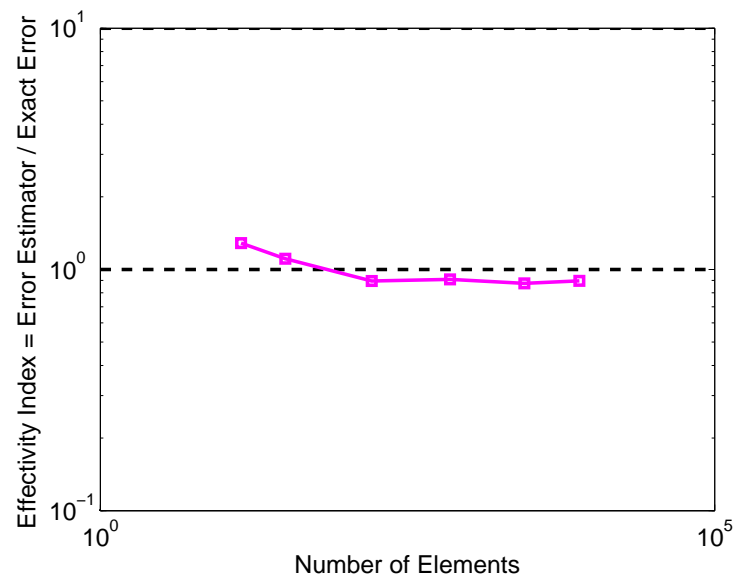


Figure 1: Effectivity index; *Left*: the Morley element (with C-boundaries); *Right*: the stabilized method (with C/S/F-boundaries).

Simply supported L-domain — Starting mesh — Deflection

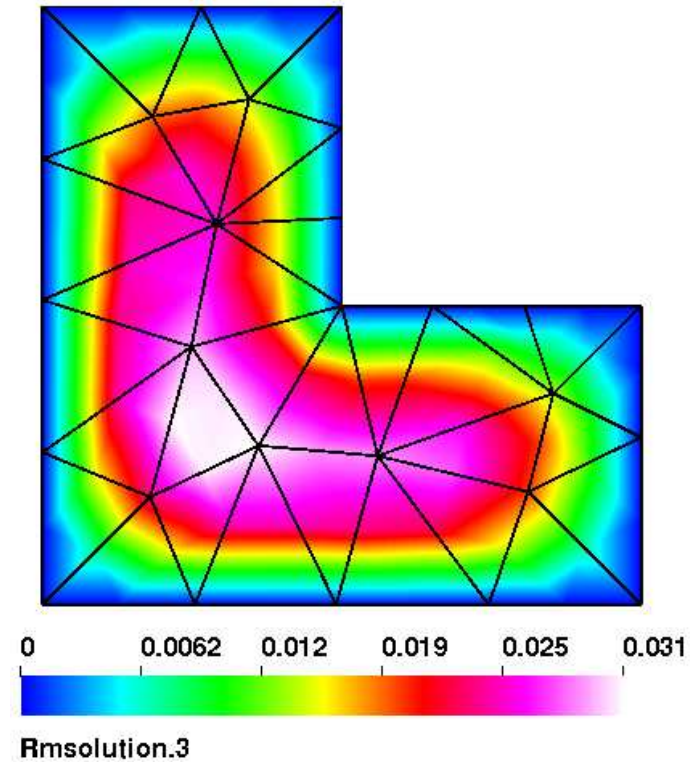


Figure 2: The **stabilized** method: Deflection distribution for the first mesh (constant loading).

Adaptively refined mesh — Error estimator

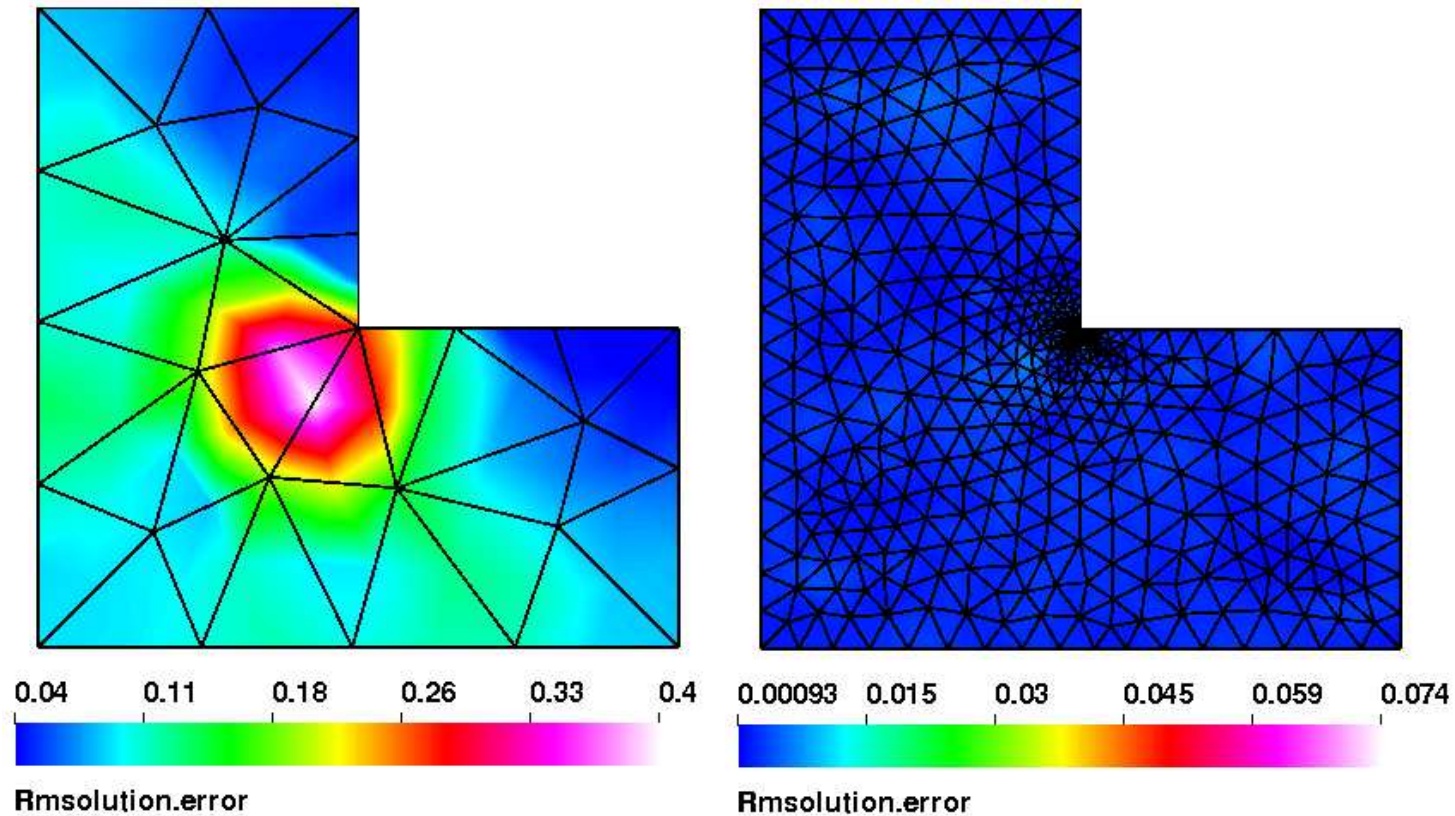


Figure 3: The **stabilized** method: Distribution of the error estimator for two adaptive steps.

Uniform vs. Adaptive — Convergence in the norm $\|\beta - \beta_h\|_1 + |(w - w_h, \beta - \beta_h)|_h$

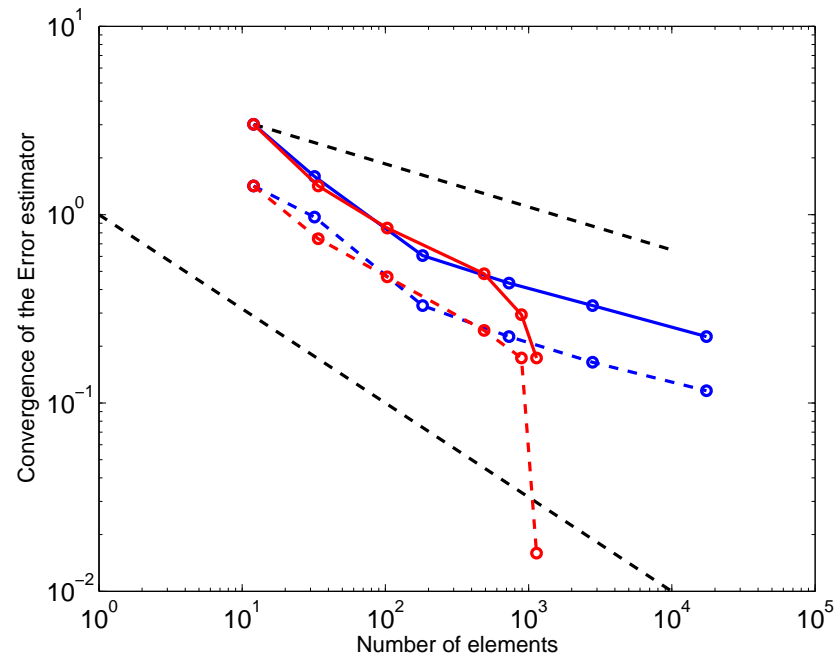


Figure 4: The **stabilized** method: Convergence of the error estimator for the **uniform refinements** and **adaptive refinements**; *Solid* lines for *global*, *dashed* lines for *maximum local* ones.

Clamped L-domain — Refinements — Error estimator

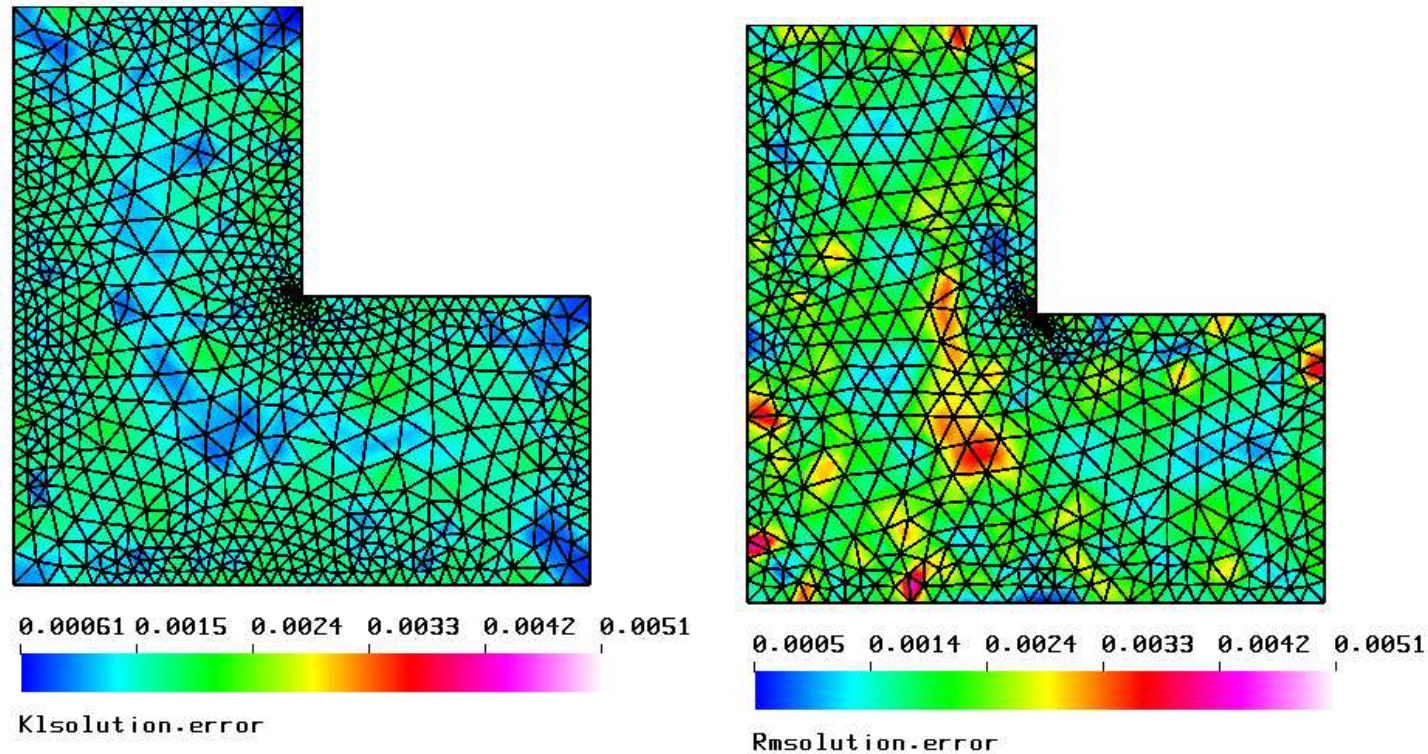


Figure 5: Distribution of the error estimator after adaptive refinements: *Left*: the **Morley** element; *Right*: the **stabilized** method.

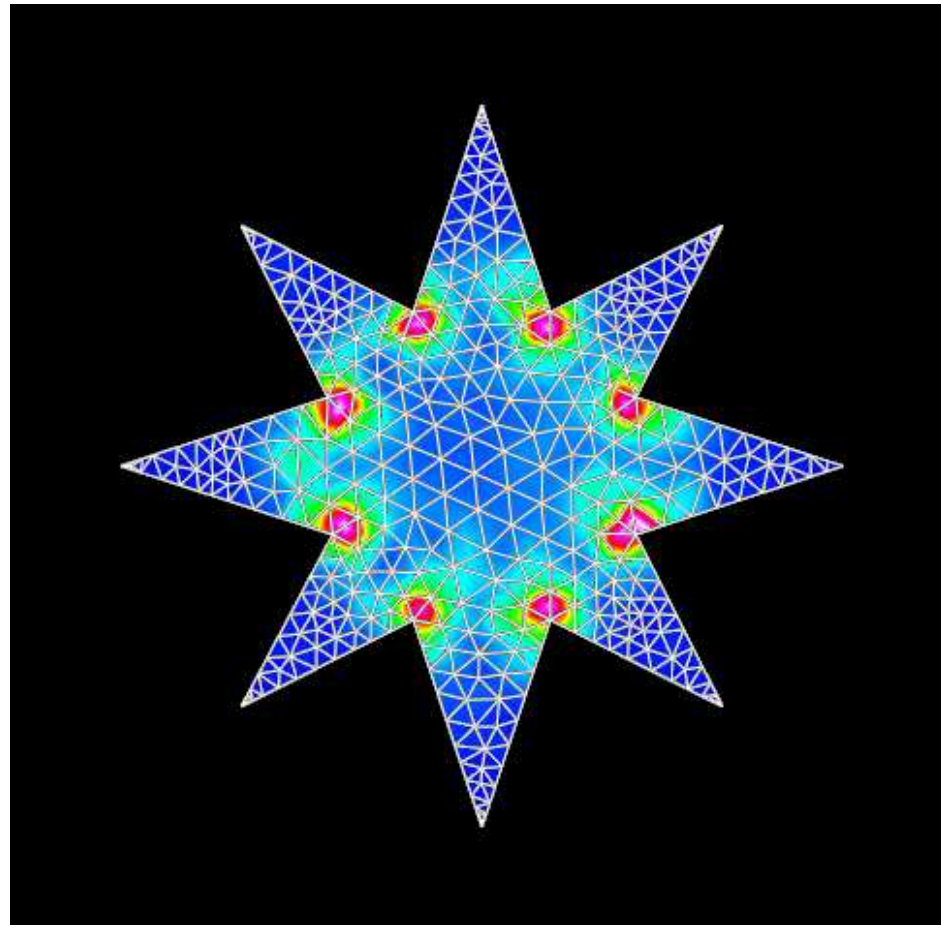
Conclusions

- ▶ A posteriori error analysis has been accomplished for Kirchhoff plates:
 - the Morley element for clamped boundaries
 - the stabilized C^0 -continuous element for general boundary conditions
 - efficient and reliable error estimators for both methods.
- ▶ Numerical benchmarks confirm the adaptive performance and robustness of the error indicators.

References

- [1] L. Beirão da Veiga, J. Niiranen, R. Stenberg: [A posteriori error estimates for the Morley plate bending element](#); *Numerische Mathematik*, 106, 165–179 (2007).
- [2] L. Beirão da Veiga, J. Niiranen, R. Stenberg: [A family of \$C^0\$ finite elements for Kirchhoff plates I: Error analysis](#); accepted for publication in *SIAM Journal on Numerical Analysis* (2007).
- [3] L. Beirão da Veiga, J. Niiranen, R. Stenberg: [A family of \$C^0\$ finite elements for Kirchhoff plates II: Numerical results](#); accepted for publication in *Computer Methods in Applied Mechanics and Engineering* (2007).

How do we deal with a blinking star?



We show it by adaptively refined
mesh flakes!

